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Duvauchelle Works Yard Workshop
PRO 3612 B002
Detailed Engineering Evaluation
Qualitative Report
Version FINAL

Pawsons Valley Road, Duvauchelle

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Pawsons Valley Road, Duvauchelle

Christchurch City Council

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Date
24/01/14

Contents

Qualitative Report Summary	1
1. Background	2
2. Compliance	3
2.1 Canterbury Earthquake Recovery Authority (CERA)	3
2.2 Building Act	4
2.3 Christchurch City Council Policy	5
2.4 Building Code	5
3. Earthquake Resistance Standards	6
4. Building Description	8
4.1 General	8
4.2 Gravity Load Resisting System	8
4.3 Lateral Load Resisting System	8
5. Assessment	10
6. Damage Assessment	11
6.1 Surrounding Buildings	11
6.2 Residual Displacements and General Observations	11
6.3 Ground Damage	11
7. Critical Structural Weakness	12
7.1 Short Columns	12
7.2 Lift Shaft	12
7.3 Roof	12
7.4 Plan Irregularity	12
7.5 Staircases	12
7.6 Liquefaction	12
8. Geotechnical Consideration	13
8.1 Site Description	13
8.2 Published Information on Ground Conditions	13
8.2.1 Environment Canterbury Logs	13
8.2.2 EQC Geotechnical Investigations	14

8.2.3	Land Zoning	14
8.2.4	Post February Aerial Photography	14
8.2.5	Summary of Ground Conditions	14
8.3	Seismicity	14
8.4	Ground Shaking Hazard	15
8.5	Slope Failure and / or Rockfall Potential	15
8.6	Liquefaction Potential	15
8.7	Recommendations	15
8.8	Conclusions & Summary	15
9.	Survey	16
10.	Initial Capacity Assessment	17
10.1	% NBS Assessment	17
10.2	Seismic Parameters	17
10.3	Expected Structural Ductility Factor	17
10.4	Discussion of Results	17
10.5	Occupancy	18
11.	Initial Conclusions	19
12.	Recommendations	20
13.	Limitations	21
13.1	General	21
13.2	Geotechnical Limitations	21

Table Index

Table 1	%NBS compared to relative risk of failure	7
Table 2	ECan Bore Log Summary Table	13
Table 3	Summary of Known Active Faults	14
Table 4	Indicative Building and Critical Structural Weaknesses Capacities based on the NZSEE Initial Evaluation Procedure	17

Figure Index

Figure 1 NZSEE Risk Classifications Extracted from table 2.2 of the NZSEE 2006 AISPBE	6
Figure 2 Plan Sketch Showing Key Structural Elements	9

Appendices

- A Photographs
- B Existing Drawings
- C CERA Building Evaluation Form

Qualitative Report Summary

Duvauchelle Works Yard Workshop

PRO 3612 B002

Detailed Engineering Evaluation

Qualitative Report - SUMMARY

Version FINAL

Pawsons Valley Road, Duvauchelle

Background

This is a summary of the Qualitative report for the building structure, and is based in general on the Detailed Engineering Evaluation Procedure document (draft) issued by the Structural Advisory Group on 19 July 2011 and visual inspections on 26 January 2012.

Key Damage Observed

No damage was observed to the building.

Critical Structural Weaknesses

The front western wall of the structure is lacking stiffness and bracing due to the open door entrances and as a result this may cause torsion. The lack of bracing considerably weakens the building. However, due to the lightweight nature of the building, the plan irregularity is not believed likely to collapse and pose a life safety hazard.

Indicative Building Strength (from IEP and CSW assessment)

Based on the information available, and using the NZSEE Initial Evaluation Procedure, the buildings original capacity has been assessed to be in the order of 45% NBS. The buildings post-earthquake capacity excluding critical structural weaknesses is in the order of 45% NBS.

The building has been assessed to have a seismic capacity in the order of 45% NBS and is potentially Earthquake Risk.

Recommendations

As the building has been assessed to achieve greater than 34% NBS, it is not considered to be an Earthquake Prone structure. It is not considered to have any critical structural weaknesses or any immediate collapse hazards therefore general occupancy of the building can continue.

1. Background

GHD has been engaged by the Christchurch City Council (CCC) to undertake a detailed engineering evaluation of the Duvauchelle Works Yard Workshop.

This report is a Qualitative Assessment of the building structure, and is based in general on the Detailed Engineering Evaluation Procedure document (draft) issued by the Structural Advisory Group on 19 July 2011.

A qualitative assessment involves inspections of the building and a desktop review of existing structural and geotechnical information, including existing drawings and calculations, if available.

The purpose of the assessment is to determine the likely building performance and damage patterns, to identify any potential critical structural weaknesses or collapse hazards, and to make an initial assessment of the likely building strength in terms of percentage of new building standard (%NBS).

At the time of this report, no intrusive site investigation, detailed analysis, or modelling of the building structure had been carried out. The building description is based on the visual inspection carried out on site and the building drawings made available.

2. Compliance

This section contains a brief summary of the requirements of the various statutes and authorities that control activities in relation to buildings in Christchurch at present.

2.1 Canterbury Earthquake Recovery Authority (CERA)

CERA was established on 28 March 2011 to take control of the recovery of Christchurch using powers established by the Canterbury Earthquake Recovery Act enacted on 18 April 2011. This act gives the Chief Executive Officer of CERA wide powers in relation to building safety, demolition and repair. Two relevant sections are:

Section 38 – Works

This section outlines a process in which the chief executive can give notice that a building is to be demolished and if the owner does not carry out the demolition, the chief executive can commission the demolition and recover the costs from the owner or by placing a charge on the owners' land.

Section 51 – Requiring Structural Survey

This section enables the chief executive to require a building owner, insurer or mortgagee carry out a full structural survey before the building is re-occupied.

We understand that CERA will require a detailed engineering evaluation to be carried out for all buildings (other than those exempt from the Earthquake Prone Building definition in the Building Act). It is anticipated that CERA will adopt the Detailed Engineering Evaluation Procedure document (draft) issued by the Structural Advisory Group on 19 July 2011. This document sets out a methodology for both qualitative and quantitative assessments.

The qualitative assessment is a desk-top and site inspection assessment. It is based on a thorough visual inspection of the building coupled with a review of available documentation such as drawings and specifications. The quantitative assessment involves analytical calculation of the buildings strength and may require non-destructive or destructive material testing, geotechnical testing and intrusive investigation.

It is anticipated that factors determining the extent of evaluation and strengthening level required will include:

- ▶ The importance level and occupancy of the building
- ▶ The placard status and amount of damage
- ▶ The age and structural type of the building
- ▶ Consideration of any critical structural weaknesses
- ▶ The extent of any earthquake damage

2.2 Building Act

Several sections of the Building Act are relevant when considering structural requirements:

Section 112 – Alterations

This section requires that an existing building complies with the relevant sections of the Building Code to at least the extent that it did prior to any alteration. This effectively means that a building cannot be weakened as a result of an alteration (including partial demolition).

Section 115 – Change of Use

This section requires that the territorial authority (in this case Christchurch City Council (CCC)) be satisfied that the building with a new use complies with the relevant sections of the Building Code 'as near as is reasonably practicable'. Regarding seismic capacity 'as near as reasonably practicable' has previously been interpreted by CCC as achieving a minimum of 67% NBS however where practical achieving 100% NBS is desirable. The New Zealand Society for Earthquake Engineering (NZSEE) recommend a minimum of 67% NBS.

2.2.1 Section 121 – Dangerous Buildings

The definition of dangerous building in the Act was extended by the Canterbury Earthquake (Building Act) Order 2010, and it now defines a building as dangerous if:

- ▶ In the ordinary course of events (excluding the occurrence of an earthquake), the building is likely to cause injury or death or damage to other property; or
- ▶ In the event of fire, injury or death to any persons in the building or on other property is likely because of fire hazard or the occupancy of the building; or
- ▶ There is a risk that the building could collapse or otherwise cause injury or death as a result of earthquake shaking that is less than a 'moderate earthquake' (refer to Section 122 below); or
- ▶ There is a risk that that other property could collapse or otherwise cause injury or death; or
- ▶ A territorial authority has not been able to undertake an inspection to determine whether the building is dangerous.

Section 122 – Earthquake Prone Buildings

This section defines a building as earthquake prone if its ultimate capacity would be exceeded in a 'moderate earthquake' and it would be likely to collapse causing injury or death, or damage to other property. A moderate earthquake is defined by the building regulations as one that would generate ground shaking 33% of the shaking used to design an equivalent new building.

Section 124 – Powers of Territorial Authorities

This section gives the territorial authority the power to require strengthening work within specified timeframes or to close and prevent occupancy to any building defined as dangerous or earthquake prone.

Section 131 – Earthquake Prone Building Policy

This section requires the territorial authority to adopt a specific policy for earthquake prone, dangerous and insanitary buildings.

2.3 Christchurch City Council Policy

Christchurch City Council adopted their Earthquake Prone, Dangerous and Insanitary Building Policy in 2006. This policy was amended immediately following the Darfield Earthquake of the 4th September 2010.

The 2010 amendment includes the following:

- ▶ A process for identifying, categorising and prioritising Earthquake Prone Buildings, commencing on 1 July 2012;
- ▶ A strengthening target level of 67% of a new building for buildings that are Earthquake Prone;
- ▶ A timeframe of 15-30 years for Earthquake Prone Buildings to be strengthened; and,
- ▶ Repair works for buildings damaged by earthquakes will be required to comply with the above.

The council has stated their willingness to consider retrofit proposals on a case by case basis, considering the economic impact of such a retrofit.

We anticipate that any building with a capacity of less than 33% NBS (including consideration of critical structural weaknesses) will need to be strengthened to a target of 67% NBS of new building standard as recommended by the Policy.

If strengthening works are undertaken, a building consent will be required. A requirement of the consent will require upgrade of the building to comply 'as near as is reasonably practicable' with:

- ▶ The accessibility requirements of the Building Code.
- ▶ The fire requirements of the Building Code. This is likely to require a fire report to be submitted with the building consent application.

2.4 Building Code

The building code outlines performance standards for buildings and the Building Act requires that all new buildings comply with this code. Compliance Documents published by The Department of Building and Housing can be used to demonstrate compliance with the Building Code.

After the February Earthquake, on 19 May 2011, Compliance Document B1: Structure was amended to include increased seismic design requirements for Canterbury as follows:

- ▶ Hazard Factor increased from 0.22 to 0.3 (36% increase in the basic seismic design load)
- ▶ Serviceability Return Period Factor increased from 0.25 to 0.33 (80% increase in the serviceability design loads when combined with the Hazard Factor increase)

The increase in the above factors has resulted in a reduction in the level of compliance of an existing building relative to a new building despite the capacity of the existing building not changing.

3. Earthquake Resistance Standards

For this assessment, the building’s earthquake resistance is compared with the current New Zealand Building Code requirements for a new building constructed on the site. This is expressed as a percentage of new building standard (%NBS). The new building standard load requirements have been determined in accordance with the current earthquake loading standard (NZS 1170.5:2004 Structural design actions - Earthquake actions - New Zealand).

The likely capacity of this building has been derived in accordance with the New Zealand Society for Earthquake Engineering (NZSEE) guidelines ‘Assessment and Improvement of the Structural Performance of Buildings in Earthquakes’ (AISPBE), 2006. These guidelines provide an Initial Evaluation Procedure that assesses a buildings capacity based on a comparison of loading codes from when the building was designed and currently. It is a quick high-level procedure that can be used when undertaking a Qualitative analysis of a building. The guidelines also provide guidance on calculating a modified Ultimate Limit State capacity of the building which is much more accurate and can be used when undertaking a Quantitative analysis.

The New Zealand Society for Earthquake Engineering has proposed a way for classifying earthquake risk for existing buildings in terms of %NBS and this is shown in Figure 1 below.

Description	Grade	Risk	%NBS	Existing Building Structural Performance	Improvement of Structural Performance	
					Legal Requirement	NZSEE Recommendation
Low Risk Building	A or B	Low	Above 67	Acceptable (improvement may be desirable)	The Building Act sets no required level of structural improvement (unless change in use) This is for each TA to decide. Improvement is not limited to 34%NBS.	100%NBS desirable. Improvement should achieve at least 67%NBS
Moderate Risk Building	B or C	Moderate	34 to 66	Acceptable legally. Improvement recommended		Not recommended. Acceptable only in exceptional circumstances
High Risk Building	D or E	High	33 or lower	Unacceptable (Improvement)	Unacceptable	Unacceptable

Figure 1 NZSEE Risk Classifications Extracted from table 2.2 of the NZSEE 2006 AISPBE

Table 1 compares the percentage NBS to the relative risk of the building failing in a seismic event with a 10% risk of exceedance in 50 years (i.e. 0.2% in the next year). It is noted that the current seismic risk in Christchurch results in a 6% risk of exceedance in the next year.

Percentage of New Building Standard (%NBS)	Relative Risk (Approximate)
>100	<1 time
80-100	1-2 times
67-80	2-5 times
33-67	5-10 times
20-33	10-25 times
<20	>25 times

Table 1 %NBS compared to relative risk of failure

4. Building Description

4.1 General

The workshop is located in the Duvauchelle works yard, Pawsons Valley Road, Akaroa. The original construction date of the workshop is unknown but based on site observation is estimated to be in the 1940's. The structure can be observed aerially in Photograph 8 in Appendix A.

The site slopes gradually towards the Christchurch Akaroa Road and seafront.

The building was used as a workshop but now acts as general storage for the works yard.

The structure consists of timber frame walls with a timber Sark boarding and weatherboard panel exterior, except for on the road side, south facing wall, which consists of corrugated metal cladding. An extension has been added to the north facing side of the building. Where the extension has been added a timber girder truss has been installed and this is supported by two timber posts bolted to concrete plinths. The foundation consists of a strip footing to the perimeter with concrete slab on grade to the interior. The roof consists of timber trusses with timber purlins and rafters with corrugated metal cladding.

The dimensions of the building are approximately 13 m long by 10 m wide and 4 m tall.

No plans or drawings were available for this building.

4.2 Gravity Load Resisting System

The timber framed load bearing walls act as the gravity load resisting system for the building. The timber trusses are connected directly into the supporting timber posts and internal column, these deliver the load to the external walls and in turn into the foundations through the concrete plinths. The timber column and concrete plinth can be observed in Photograph 4 in Appendix A.

4.3 Lateral Load Resisting System

The seismic load is resisted in both the transverse and longitudinal directions by the timber Sark board lining and weather board panelling on three sides of the structure. In addition the corrugated exterior will offer some lateral load resistance and there is a likelihood of timber diagonal braces existing inside the walls, this however could not be observed. The lack of diagonal bracing in the roof will result in seismic load not being efficiently transferred to the load resisting walls. The timber Sark boarding in the roof acts as a diaphragm and allows the transfer of the load to the supporting walls.

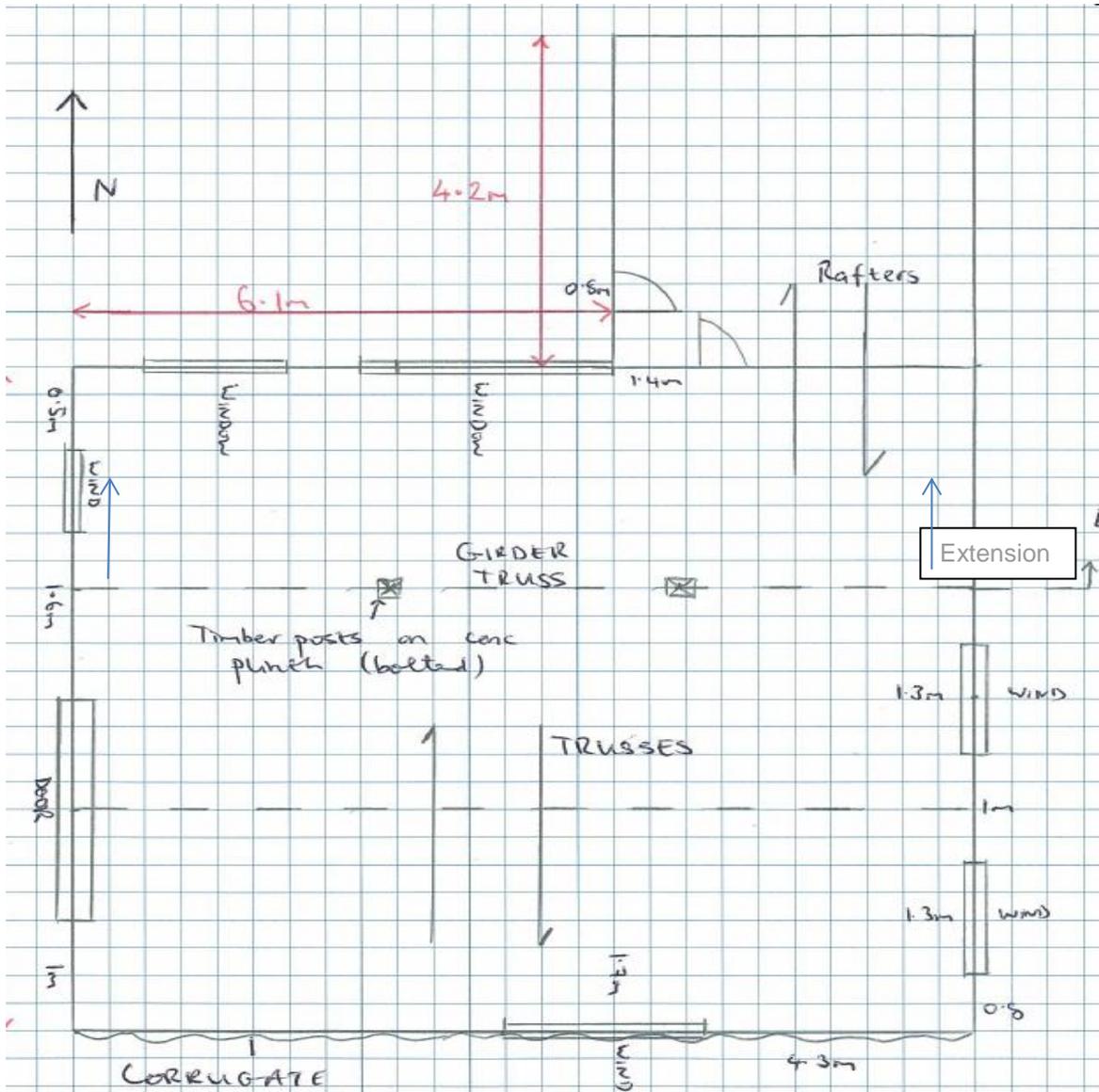


Figure 2 Plan Sketch Showing Key Structural Elements

5. Assessment

A visual inspection of the building was undertaken on 26 January 2012. Both the interior and exterior of the building were inspected. The building was observed to have a green placard in place. The main structural components of the building were able to be viewed due to the exposed construction of the building.

The visual inspection consisted of observing the building to determine the structural systems and likely behaviour of the building during an earthquake. The site was assessed for damage, including observing the ground conditions, checking for damage in areas where damage would be expected for the structure type observed and noting general damage observed throughout the building in both structural and non-structural elements.

The %NBS score is determined using the IEP procedure described by the NZSEE which is based on the information obtained from visual observation of the building.

6. Damage Assessment

6.1 Surrounding Buildings

Slight cracking was noted in the front wall of the Duvauchelle works yard fire shed nearby. In addition a neighbouring residential dwelling was observed to have minor cracking in its exterior blockwork.

6.2 Residual Displacements and General Observations

No residual displacements were noted to the building.

No damage was noted to the timber structure including the lightweight metal roof cladding.

Cracking to the perimeter strip footing was noted and can be observed in Photograph 6 in Appendix A. Existing shrinkage cracks to the concrete floor slab and perimeter wall were observed, these may have opened up slightly during the recent seismic activity.

6.3 Ground Damage

No ground damage was observed during our inspection of the site.

7. Critical Structural Weakness

7.1 Short Columns

The building does not contain any short columns.

7.2 Lift Shaft

The building does not contain a lift shaft.

7.3 Roof

No diagonal roof bracing exists consequently load transfer from the front west open side will not be effective.

7.4 Plan Irregularity

There is seismic bracing in only three of the four sides of this structure, causing torsional effects. The front of the building is open-sided and lacking bracing, so consequently bracing this side must be achieved via roof diaphragm action from the timber sarking. This will allow the transfer of the seismic load to the rear wall. Due to the lightweight nature of the building it is unlikely that it will prematurely collapse as a result of plan irregularity during a seismic event. As such, plan irregularity is considered to be "insignificant".

7.5 Staircases

The building does not contain a staircase.

7.6 Liquefaction

No liquefaction was observed on site.

8. Geotechnical Consideration

8.1 Site Description

The site is located at the Duvauchelle Bay (Akaroa Harbour) end of the Pawsons Valley Road and within a predominantly rural area of Christchurch. To the east of the road the terrain rises moderately, but to the west and south of the site the terrain is gentle sloping down to the water's edge (180m to the south). The site is at approximately 10m above mean sea level, and approximately 35km (straight line distance) from Christchurch City centre. A stream is located approximately 100m to the west of the site.

8.2 Published Information on Ground Conditions

8.2.1 Published Geology

The geological map of the area¹ indicates that the site is at the boundary of the following layers:

- Grey river alluvium, comprising gravel, sand and silt (Holocene in age); and
- Yellow-brown windblown silt (>3m thick and commonly in multiple layers) (Pleistocene in age) (commonly called Loess).

These layers are underlain by basaltic to trachytic lava flows with associated tuff and pyroclastic breccia of the Akaroa Volcanic Group.

8.2.1 Environment Canterbury Logs

Information from Environment Canterbury (ECan) indicates that there is one borehole is located within 350 m of the site. The lithology is summarised below. This log indicates the area is predominantly underlain by layers of volcanic rocks and clay (see Table 2).

Table 2 ECan Bore Log Summary Table

Bore Name	Grid Reference	Log Summary	Distance & Direction from Site
M35/9948	2504300 mE 5717500 mN	0 – 2.0 m Clay 2.0 m 2.59 m Large stones and rocks 2.59 – 5.5 m Claybound volcanic rocks 5.5 – 6.9 m Blue and brown clay 6.9 – 7.3 m Hard claybound volcanic rock	310m WSW

It should be noted that the purpose of the boreholes the well logs are associated with, were sunk for groundwater extraction and not for geotechnical purposes. Therefore, the amount of material recovered and available for interpretation and recording will have been variable at best and may not be

¹ Brown, L. J. and Weeber J.H. 1992: Geology of the Christchurch Urban Area. Institute of Geological and Nuclear Sciences 1:25,000 Geological Map 1. Lower Hutt. Institute of Geological and Nuclear Sciences Limited.

² Tonkin and Taylor . September 2011: Christchurch Earthquake Recovery, Geotechnical Factual Report, Hoon Hay.

representative. The logs have been written by the well driller and not a geotechnical professional or to a standard. In addition strength data is not recorded.

8.2.2 EQC Geotechnical Investigations

No Earthquake Commission geotechnical testing has been undertaken in this area.

8.2.3 Land Zoning

Canterbury Earthquake Recovery Authority (CERA) has published areas showing the Green Zone Technical Category in relation to the risk of future liquefaction and how these areas are expected to perform in future earthquakes. Properties in the Port Hills and Banks Peninsula have not been given a Technical Category.

8.2.4 Post February Aerial Photography

There is no post February 2011 earthquake aerial photography of this site available from Koordinates.

8.2.5 Summary of Ground Conditions

From the published and available data the site is indicated to be underlain by shallow soils comprising alluvial materials, loess and volcanic rocks.

8.3 Seismicity

8.3.1 Nearby Faults

There are many faults in the Christchurch region, however only those considered most likely to have an adverse effect on the site are detailed below.

Table 3 Summary of Known Active Faults^{2,3}

Known Active Fault	Distance from Site (km)	Max Likely Magnitude	Avg Recurrence Interval
Alpine Fault	165	~8.3	300 years
Greendale (2010) Fault	50	7.1	~15,000 years
Hope Fault	145	7.2~7.5	120~200 years
Kelly Fault	145	7.2	150 years
Porters Pass Fault	100	7.0	1100 years

Recent earthquakes since 22 February 2011 have identified the presence of a previously undetected active fault system / zone underneath Christchurch City and the Port Hills. Research and published

² Stirling, M.W, McVerry, G.H, and Berryman K.R. (2002) A New Seismic Hazard Model for New Zealand, Bulletin of the Seismological Society of America, Vol. 92 No. 5, pp 1878-1903, June 2002.

³ GNS Active Faults Database

information on this system is in development and not generally available. Average recurrence intervals are yet to be estimated.

8.4 Ground Shaking Hazard

This seismic activity has produced earthquakes of Magnitude-6.3 with peak ground accelerations (PGA) up to twice the acceleration due to gravity (2g) in some parts of the city. This has resulted in widespread liquefaction throughout Christchurch.

New Zealand Standard NZS 1170.5:2004 quantifies the Seismic Hazard factor for Akaroa as 0.16, being in a moderate to high earthquake zone. This value has been provisionally upgraded recently (from 0.22) to reflect the seismicity hazard observed in the earthquakes since 4 September 2010.

In addition, anticipation of alluvium and loess over volcanic rocks, and a 475-year PGA (peak ground acceleration) of ~0.4 (Stirling et al, 20024). Ground shaking is expected to be moderate.

8.5 Slope Failure and / or Rockfall Potential

Shallow highly saturated failures and deep seated instability to the east of the road may have the ability to impact the site. However the site itself is on predominately flat land. Rockfall is not considered to be an issue at the site based on the information available.

8.6 Liquefaction Potential

Due to the likely presence of alluvial soils and loess at the site it is considered possible that liquefaction will occur where sands and silts are present with a sufficiently high groundwater table. No signs of liquefaction were observed by the personnel inspecting the structures on the site.

8.7 Recommendations

To better clarify the material underlying the site given its proximity to the material boundaries on the map, a shallow borehole would be recommended. This may result in the liquefaction potential of the site being reduced.

8.8 Conclusions & Summary

This assessment is based on a review of the geology and existing ground investigation information, and observations from the Christchurch earthquakes since 4 September 2010.

The site appears to be situated at the boarder of loess and alluvial deposits. Associated with this the loess and fine grained alluvial deposits the site also has a moderate to high liquefaction potential.

Should a more comprehensive liquefaction and/or ground condition assessment be required, it is recommended that an intrusive investigation comprising of at least one piezocone CPT be conducted.

A soil class of C (in accordance with NZS 1170.5:2004) should be adopted for the site.

9. Survey

No level or verticality surveys have been undertaken for this building at this stage.

10. Initial Capacity Assessment

10.1 % NBS Assessment

Following an IEP assessment, the building has been assessed as achieving 16% New Building Standard (NBS). Under the New Zealand Society for Earthquake Engineering (NZSEE) guidelines the building is considered potentially Earthquake Prone in terms of the building act as it achieves below 33% NBS.

<u>Item</u>	<u>%NBS</u>
Building excluding CSW's	45

Table 4 Indicative Building and Critical Structural Weaknesses Capacities based on the NZSEE Initial Evaluation Procedure

10.2 Seismic Parameters

The seismic design parameters based on current design requirements from NZS1170:2002 and the NZBC clause B1 for this building are:

- ▶ Site soil class: D, NZS 1170.5:2004, Clause 3.1.3, Soft Soil
- ▶ Site hazard factor, $Z = 0.3$, NZBC, Clause B1 Structure, Amendment 11 effective from 1 August 2011
- ▶ Return period factor $R_u = 0.5$, NZS 1170.5:2004, Table 3.5, Importance Level 1 structure with a 50 year design life.

Several key seismic parameters have influenced the %NBS score obtained from the IEP assessment. The building has been assessed as an Importance Level 1 building. An increased Z factor of 0.3 for Christchurch has been used in line with recommendations from the Department of Building and Housing recommendations resulting in a reduced % NBS score.

10.3 Expected Structural Ductility Factor

A structural ductility factor of 2.0 has been assumed longitudinally and transversely based on the timber framed and panelled structure.

10.4 Discussion of Results

The results obtained from the initial IEP assessment are consistent with those expected for a building of this age, importance level and construction type founded on Class D soils. This building would have been designed to standards currently out of date. These standards would have used design loads significantly less than those required by the current loading standard and detailing requirements for ductile seismic behaviour that are present in the current standards. In addition the hazard factor for Christchurch has increased to 0.3. The building was estimated to be an Importance Level 1 building, hence the Return Period Scaling Factor is increased to 2.0, this results in an increased %NBS. It is reasonable to expect the building to be classified as Earthquake Risk.

10.5 Occupancy

As the building has been assessed to have a %NBS not exceeding 33% NBS, it is deemed as potentially Earthquake Prone. As a result, it is recommended that the structure is unoccupied pending further detailed assessment and strengthening if required, as per Christchurch City Council's policy regarding occupancy of potentially Earthquake Prone buildings.

11. Initial Conclusions

As the structure achieves 45% NBS, it is not deemed a potentially Earthquake Prone structure in accordance with the NZSEE guidelines. Additionally, there was no observed damage to the lateral load resisting system. The building does not pose an immediate risk to users and occupants as no collapse hazards have been identified, therefore general occupancy of the building is recommended.

12. Recommendations

No damage to the building was observed during the recent seismic activity in Christchurch.

As the building has achieved greater than 33% NBS following an initial IEP assessment it is regarded as Earthquake Risk. As a result, we recommend that further detailed assessment of the structure is not necessary.

13. Limitations

13.1 General

This report has been prepared subject to the following limitations:

- ▶ No intrusive structural investigations have been undertaken.
- ▶ No intrusive geotechnical investigations have been undertaken.
- ▶ No level or verticality surveys have been undertaken.
- ▶ No material testing has been undertaken.
- ▶ No calculations, other than those included as part of the IEP in the CERA Building Evaluation Report, have been undertaken. No modelling of the building for structural analysis purposes has been performed.

It is noted that this report has been prepared at the request of Christchurch City Council and is intended to be used for their purposes only. GHD accepts no responsibility for any other party or person who relies on the information contained in this report.

13.2 Geotechnical Limitations

This report presents the results of a geotechnical appraisal prepared for the purpose of this commission, and for prepared solely for the use of Christchurch City Council and their advisors. The data and advice provided herein relate only to the project and structures described herein and must be reviewed by a competent geotechnical engineer before being used for any other purpose. GHD Limited (GHD) accepts no responsibility for other use of the data.

The advice tendered in this report is based on a visual geotechnical appraisal. No subsurface investigations have been conducted. An assessment of the topographical land features have been made based on this information. It is emphasised that Geotechnical conditions may vary substantially across the site from where observations have been made. Subsurface conditions, including groundwater levels can change in a limited distance or time. In evaluation of this report cognisance should be taken of the limitations of this type of investigation.

An understanding of the geotechnical site conditions depends on the integration of many pieces of information, some regional, some site specific, some structure specific and some experienced based. Hence this report should not be altered, amended or abbreviated, issued in part and issued incomplete in any way without prior checking and approval by GHD. GHD accepts no responsibility for any circumstances, which arise from the issue of the report, which have been modified in any way as outlined above.

Appendix A
Photographs



Photograph 1: West (front) Elevation



Photograph 2: North Elevation



Photograph 3: South roadside Elevation with metal cladding



Photograph 4: Interior timber panelling/ Timber column with concrete plinth



Photograph 5: Slab on grade interior floor



Photograph 6: Existing cracking to the perimeter strip footing may have opened up further in recent seismic activity



Photograph 7: Timber framed roof with timber gurder truss



Photograph 8: Aerial Plan with Arrow pointing to Workshop Structure

Appendix B
Existing Drawings/Sketches

Appendix C
CERA Building Evaluation Form

Detailed Engineering Evaluation Summary Data

V1.11

Location

Building Name:

Unit No: Street

Building Address:

Legal Description:

	Degrees	Min	Sec
GPS south:	43	45	0.78
GPS east:	172	56	5.00

Building Unique Identifier (CCC):

Reviewer:

CPEng No:

Company:

Company project number:

Company phone number:

Date of submission:

Inspection Date:

Revision:

Is there a full report with this summary?

Site

Site slope:

Soil type:

Site Class (to NZS1170.5):

Proximity to waterway (m, if <100m):

Proximity to cliff top (m, if < 100m):

Proximity to cliff base (m,if <100m):

Max retaining height (m):

Soil Profile (if available):

If Ground improvement on site, describe:

Approx site elevation (m):

Building

No. of storeys above ground:

Ground floor split?

Storeys below ground:

Foundation type:

Building height (m):

Floor footprint area (approx):

Age of Building (years):

Strengthening present?

Use (ground floor):

Use (upper floors):

Use notes (if required):

Importance level (to NZS1170.5):

single storey = 1

Ground floor elevation (Absolute) (m):

Ground floor elevation above ground (m):

if Foundation type is other, describe:

height from ground to level of uppermost seismic mass (for IEP only) (m):

Date of design:

If so, when (year)?

And what load level (%g)?

Brief strengthening description:

Gravity Structure

Gravity System:

Roof:

Floors:

Beams:

Columns:

Walls:

truss depth, purlin type and cladding:

slab thickness (mm):

type:

typical dimensions (mm x mm):

Lateral load resisting structure

Lateral system along:
Ductility assumed, μ :
Period along:
Total deflection (ULS) (mm):
maximum interstorey deflection (ULS) (mm):

0.00

Note: Define along and across in detailed report!

describe system

estimate or calculation?

estimate or calculation?

Lateral system across:
Ductility assumed, μ :
Period across:
Total deflection (ULS) (mm):
maximum interstorey deflection (ULS) (mm):

0.00

describe system

estimate or calculation?

estimate or calculation?

Separations:

north (mm):
east (mm):
south (mm):
west (mm):

leave blank if not relevant

Non-structural elements

Stairs:
Wall cladding:
Roof Cladding:
Glazing:
Ceilings:
Services(list):

describe

describe

Available documentation

Architectural:
Structural:
Mechanical:
Electrical:
Geotech report:

original designer name/date

Damage

Site:
(refer DEE Table 4-2)

Site performance:

Describe damage:

Settlement:
Differential settlement:
Liquefaction:
Lateral Spread:
Differential lateral spread:
Ground cracks:
Damage to area:

notes (if applicable):

Building: Current Placard Status:

Along Damage ratio: Describe how damage ratio arrived at:

Describe (summary):

Across Damage ratio: $Damage_Ratio = \frac{(\%NBS(before) - \%NBS(after))}{\%NBS(before)}$

Describe (summary):

Diaphragms Damage?: Describe:

CSWs: Damage?: Describe:

Pounding: Damage?: Describe:

Non-structural: Damage?: Describe:

Recommendations

Level of repair/strengthening required:

Describe:

Building Consent required:

Describe:

Interim occupancy recommendations:

Describe:

Along Assessed %NBS before: 45% %NBS from IEP below If IEP not used, please detail assessment methodology:

Assessed %NBS after:

Across Assessed %NBS before: 45% %NBS from IEP below

Assessed %NBS after:

IEP Use of this method is not mandatory - more detailed analysis may give a different answer, which would take precedence. Do not fill in fields if not using IEP.

Period of design of building (from above): 1935-1965 h_n from above: 4.1m

Seismic Zone, if designed between 1965 and 1992: not required for this age of building

not required for this age of building

	along	across
Period (from above):	0.1	0.1
(%NBS) _{nom} from Fig 3.3:	3.0%	3.0%
Note:1 for specifically design public buildings, to the code of the day: pre-1965 = 1.25; 1965-1976, Zone A = 1.33; 1965-1976, Zone B = 1.2; all else 1.0		1.00
Note 2: for RC buildings designed between 1976-1984, use 1.2		1.0
Note 3: for buildings designed prior to 1935 use 0.8, except in Wellington (1.0)		1.0
Final (%NBS)_{nom}:	3%	3%

2.2 Near Fault Scaling Factor

Near Fault scaling factor, from NZS1170.5, cl 3.1.6:

	along	across
Near Fault scaling factor (1/N(T,D), Factor A:	1	1

2.3 Hazard Scaling Factor

Hazard factor Z for site from AS1170.5, Table 3.3:	0.30
Z ₁₉₉₂ , from NZS4203:1992	0.8
Hazard scaling factor, Factor B:	3.33333333

2.4 Return Period Scaling Factor

Building Importance level (from above):	1
Return Period Scaling factor from Table 3.1, Factor C:	2.00

2.5 Ductility Scaling Factor

Assessed ductility (less than max in Table 3.2) Ductility scaling factor: =1 from 1976 onwards; or =k _μ , if pre-1976, from Table 3.3:	along	across
	2.00	2.00
Ductility Scaling Factor, Factor D:	1.57	1.57

2.6 Structural Performance Scaling Factor:

Sp:	0.700	0.700
Structural Performance Scaling Factor Factor E:	1.428571429	1.428571429

2.7 Baseline %NBS, (NBS%)_b = (%NBS)_{nom} x A x B x C x D x E

%NBS:	45%	45%
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Global Critical Structural Weaknesses: (refer to NZSEE IEP Table 3.4)

3.1. Plan Irregularity, factor A: insignificant 1

3.2. Vertical irregularity, Factor B: insignificant 1

3.3. Short columns, Factor C: insignificant 1

3.4. Pounding potential
Pounding effect D1, from Table to right 1.0
Height Difference effect D2, from Table to right 1.0

Therefore, Factor D: 1

3.5. Site Characteristics insignificant 1

Table for selection of D1	Severe	Significant	Insignificant/none
	Separation 0<sep<.005H	.005<sep<.01H	Sep>.01H
Alignment of floors within 20% of H	0.7	0.8	1
Alignment of floors not within 20% of H	0.4	0.7	0.8

Table for Selection of D2	Severe	Significant	Insignificant/none
	Separation 0<sep<.005H	.005<sep<.01H	Sep>.01H
Height difference > 4 storeys	0.4	0.7	1
Height difference 2 to 4 storeys	0.7	0.9	1
Height difference < 2 storeys	1	1	1

3.6. Other factors, Factor F

For ≤ 3 storeys, max value =2.5, otherwise max value =1.5, no minimum
Rationale for choice of F factor, if not 1

Along	Across
1.0	1.0

Detail Critical Structural Weaknesses: (refer to DEE Procedure section 6)

List any: Refer also section 6.3.1 of DEE for discussion of F factor modification for other critical structural weaknesses

3.7. Overall Performance Achievement ratio (PAR)

1.00	1.00
------	------

4.3 PAR x (%NBS)_b:

PAR x Baseline %NBS:	45%	45%
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4.4 Percentage New Building Standard (%NBS), (before)

45%

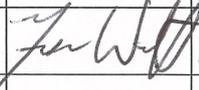
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