



## Appendix R

# Stormwater Design Memorandum

# DESIGN ADVICE MEMO

## CIVIL



Memo No **C01**  
Job Name **Papanui Pak'nSave**  
Job No **171259/C/1**  
Date **4 April 2019**  
To **Aurecon**  
Email **Michelle.Ruske@aurecongroup.com**  
Attention **Michelle Ruske**  
Copies to Client **Foodstuffs: Rebecca Parish**  
LA **Rough & Milne : Sophie Strachan**  
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A handwritten signature in blue ink, appearing to read 'Keegan Brogden', written over a light blue horizontal line.

Signature **Keegan Brogden**

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## Preliminary Pavement Catchments and First Flush Treatment for Papanui Pak'nSave

### 1. General

This memo summarises preliminary pavement catchments and stormwater first flush treatment methods for the proposed Papanui Pak'nSave development. Refer to the attached Sketch SK C39

### 2. Stormwater Outfalls

There are two stormwater outfalls serving the site. Both head towards Main North Road, east of the proposed development.

One outfall is an existing 750Ø pipe south of the proposed development.

The second will be a new 750Ø pipe proposed to replace the existing boxed Lydia Drain. Both pipes (will) have relatively shallow inverts approximately 1.5m below existing ground level at the east site boundary.

### 3. Catchments

The preliminary stormwater catchments on C39 were determined by assessing proposed future finished levels, available pipe outfall locations and depths, and appropriate areas for proposed treatment devices.

The design intention for catchment areas was to maximise hardstand stormwater treatment area directed to infiltration basins and proprietary raingardens, and if not achievable to proprietary filter systems.

All catchments are approximate & will be refined during developed design.

### 4. Proposed Stormwater Treatment Methods

There are three proposed methods for stormwater treatment where possible. In order of preference these are:

- a. Infiltration basins
- b. Proprietary raingardens (Stormwater 360 Filterra)
- c. Proprietary filter system (Stormwater 360 Stormfilters)

The design intent for treatment devices is that they have downstream hydraulic driving head higher than the top of the outlet pipes. That is, assuming the 750Ø pipes outlet pipes are running full.

### a. Infiltration Basins

Two infiltration basin systems are proposed, Basin 1A & 1B (combined) & Basin 2, both designed to treat 25mm min first flush volume. The basins will be vegetated and have an engineered high permeability base. Cleaned stormwater will be collected by underdrains and discharge to the northern 750Ø outlet. Water quantities greater than first flush volume will discharge directly into the northern 750mmØ pipe outlet. Final storage volumes and catchments will be refined at during developed design.

Basin 1A & Basin 1B are connected by pipe, for a combined 40m<sup>3</sup> first flush storage volume. Catchment B (1600m<sup>2</sup>) contributes to this basin system. For 25mm first flush volume the maximum area is 1600m<sup>2</sup>.

Basin 2 contains max 50m<sup>3</sup> first flush storage volume. Area E (1870m<sup>3</sup>) is currently proposed to discharge to Basin 2. For 25mm first flush volume the maximum contributing area is 2000m<sup>2</sup> so this basin & catchment will be refined during developed design to provide a practical balance between piperuns, and final the final basin footprint area

### b. Proprietary Raingardens

For this site, raingardens are preferred over filtration type treatment systems due to their greater ability to trap metal contaminants.

Where available landscape space is too small for stormwater basins and proposed finished levels allow, proprietary raingardens, (“Stormwater 360 Filterra”), are proposed for stormwater treatment for rain events up to 5mm/hr.

Two catchments are proposed to be treated by Filterra’s, Area F (3300m<sup>2</sup>) & Area G (1030m<sup>2</sup>):

- Two Filterra units end to end are proposed for Area F, both located in the centre of the main carpark area within a narrow garden beside a footpath.
- A single Filterra unit located near the future sign foundation is proposed to treat stormwater runoff from the south vehicle entry and pumped stormwater originating from the basement ramp.

Both Filterra locations discharge to the southern 750Ø outlet pipe from the site.

### c. Proprietary Filter Systems

For Area C (1900m<sup>2</sup>) there is no landscape space available but proposed raised ground levels will allow a proprietary stormwater filter system (“Stormwater 360 Stormfilter”) to be used to treat stormwater.

## 5. Untreated Hardstand Areas

Areas A,D, and an existing retail site are not proposed to receive first flush treatment.

Area A, (1100m<sup>2</sup>) is an existing driveway that will receive minimal changes to levels due to the proposed development. There is an existing sump collecting this catchment and the receiving existing 525mmØ pipework is too shallow to intercept the sump to provide a stormwater treatment device.

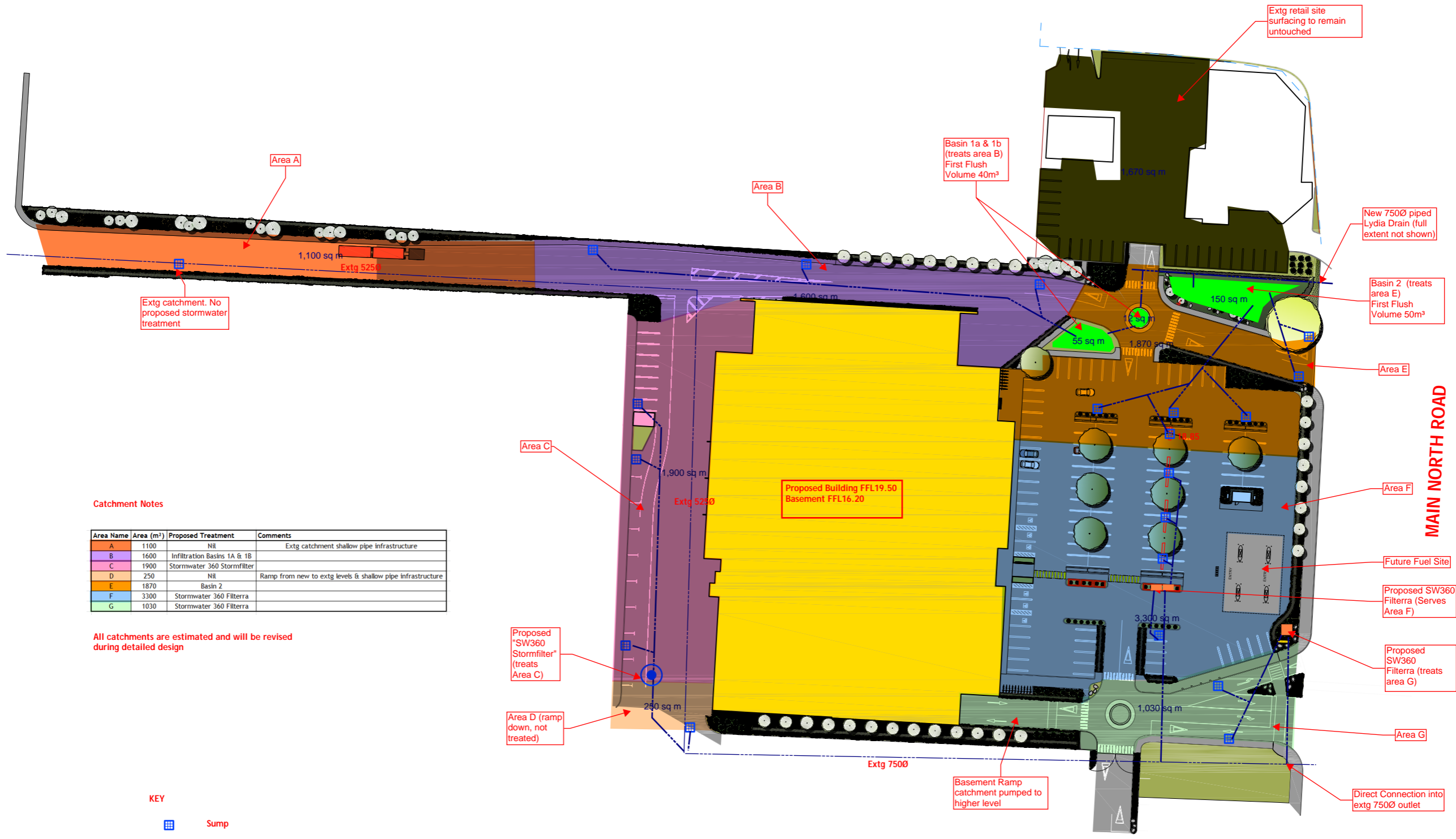
Area D, (250m<sup>2</sup>) is ramping from existing to higher proposed levels in the yard area behind the new supermarket. The depth to the invert of the receiving 750Ø that terminates in the area is too shallow for this catchment to be treated. An oil/grit interceptor providing a coarse level of treatment may be possible for this catchment prior to discharge to the council infrastructure.

An existing retail site (1670m<sup>2</sup>) currently discharges directly into Lydia Drain without treatment. No changes to the levels at this site are proposed and the depth to the outlet does not allow for a treatment device to be installed.

NORTHCOTE ROAD

LYDIA STREET

MAIN NORTH ROAD



Catchment Notes

Area Name	Area (m²)	Proposed Treatment	Comments
A	1100	Nil	Extg catchment shallow pipe infrastructure
B	1600	Infiltration Basins 1A & 1B	
C	1900	Stormwater 360 Stormfilter	
D	250	Nil	Ramp from new to extg levels & shallow pipe infrastructure
E	1870	Basin 2	
F	3300	Stormwater 360 Filterra	
G	1030	Stormwater 360 Filterra	

All catchments are estimated and will be revised during detailed design

- KEY**
- Sump
  - Vee channel
  - Proposed new SW Pipework
  - Extg SW Pipework
  - Proposed "Stormwater 360 Filterra"
  - Proposed "Stormwater 360 Stormfilter"
  - Proposed Infiltration Basin

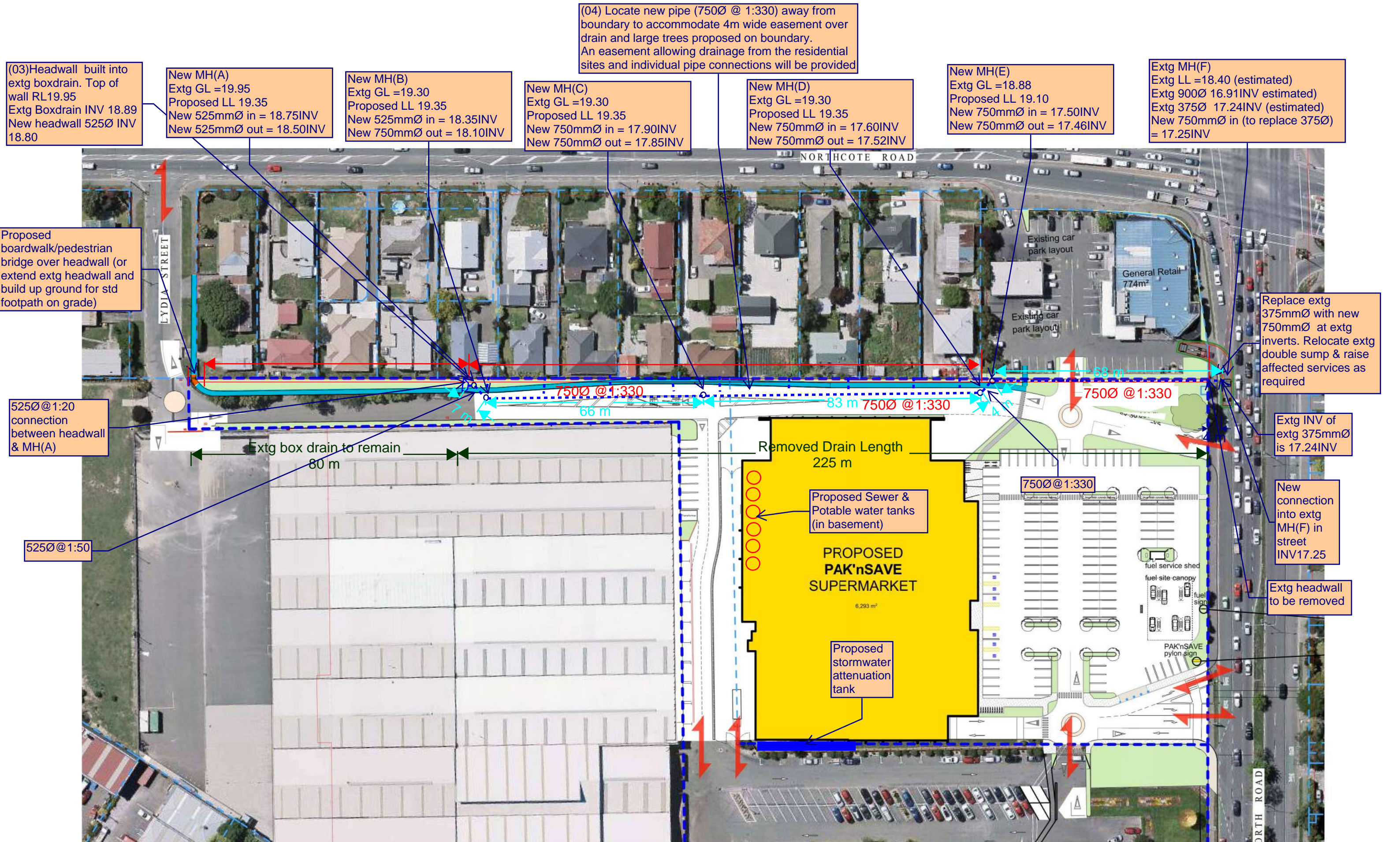


Project name : Papanui Pak'nSave Date : 04 Apr 2019  
 Job no. : 171259/C/2 Scale : 1:1000 @ A3  
 Sketch title : Preliminary Drawn : KGB  
 Pavement Catchments & Proposed Stormwater Treatment Sketch : SK C39



## Appendix S

# Lydia Street Drain Piping and Easement Plan



(04) Locate new pipe (750Ø @ 1:330) away from boundary to accommodate 4m wide easement over drain and large trees proposed on boundary. An easement allowing drainage from the residential sites and individual pipe connections will be provided

(03) Headwall built into extg boxdrain. Top of wall RL19.95  
Extg Boxdrain INV 18.89  
New headwall 525Ø INV 18.80

New MH(A)  
Extg GL =19.95  
Proposed LL 19.35  
New 525mmØ in = 18.75INV  
New 525mmØ out = 18.50INV

New MH(B)  
Extg GL =19.30  
Proposed LL 19.35  
New 525mmØ in = 18.35INV  
New 750mmØ out = 18.10INV

New MH(C)  
Extg GL =19.30  
Proposed LL 19.35  
New 750mmØ in = 17.90INV  
New 750mmØ out = 17.85INV

New MH(D)  
Extg GL =19.30  
Proposed LL 19.35  
New 750mmØ in = 17.60INV  
New 750mmØ out = 17.52INV

New MH(E)  
Extg GL =18.88  
Proposed LL 19.10  
New 750mmØ in = 17.50INV  
New 750mmØ out = 17.46INV

Extg MH(F)  
Extg LL =18.40 (estimated)  
Extg 900Ø 16.91INV estimated  
Extg 375Ø 17.24INV (estimated)  
New 750mmØ in (to replace 375Ø) = 17.25INV

Proposed boardwalk/pedestrian bridge over headwall (or extend extg headwall and build up ground for std footpath on grade)

525Ø@1:20 connection between headwall & MH(A)

525Ø@1:50

Extg box drain to remain 80 m

Removed Drain Length 225 m

Proposed Sewer & Potable water tanks (in basement)

PROPOSED PAK'nSAVE SUPERMARKET  
6,293 m²

Proposed stormwater attenuation tank

750Ø@1:330

Replace extg 375mmØ with new 750mmØ with extg inverters. Relocate extg double sump & raise affected services as required

Extg INV of extg 375mmØ is 17.24INV

New connection into extg MH(F) in street INV17.25

Extg headwall to be removed

Key

- Proposed Boardwalk
- Proposed Asphalt or Concrete Footpath

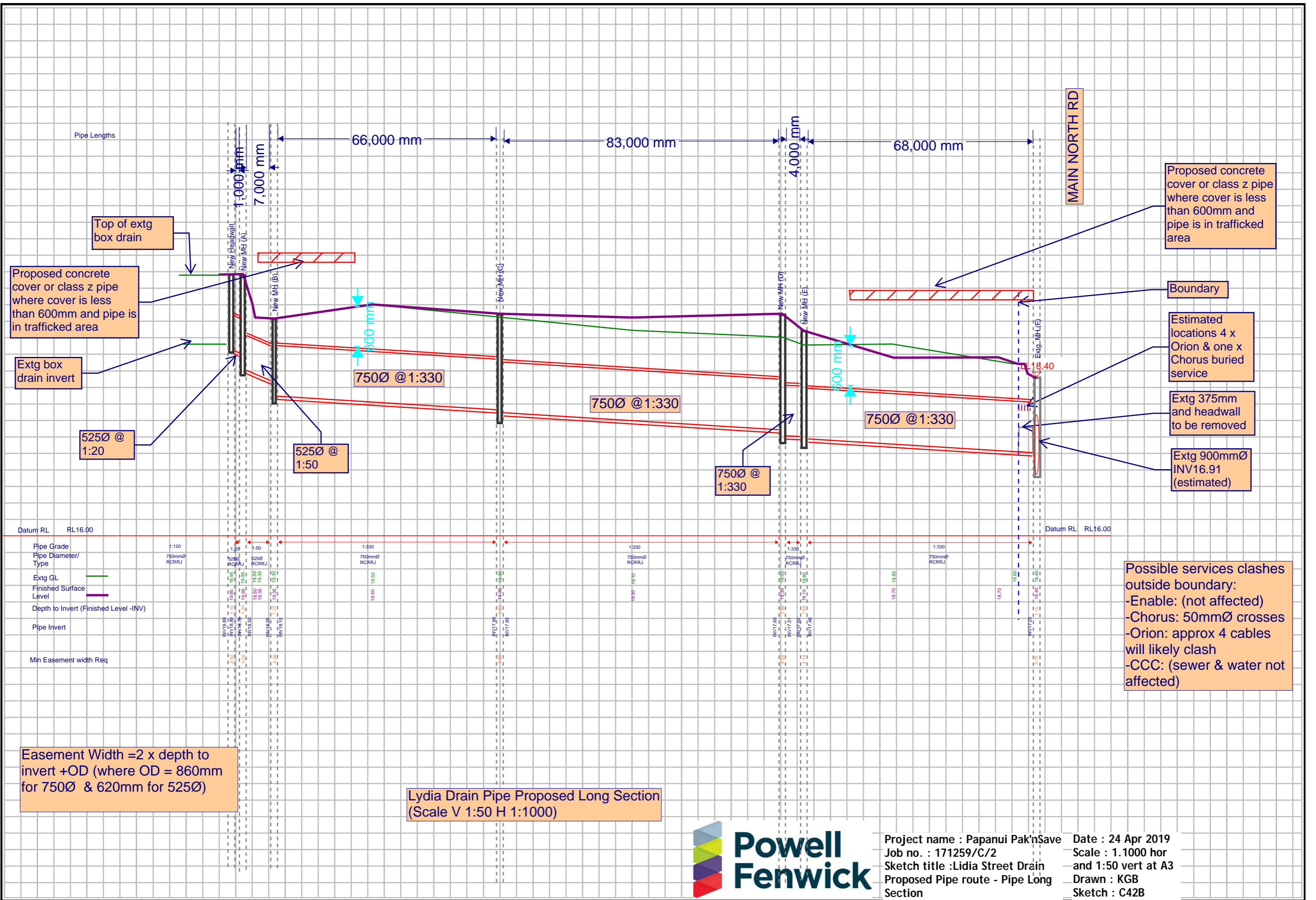
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Proposed New Pipework

**Minimum Piping Option:** Removes 225 m (approx) of extg drain  
Total Extg Open Drain Length in Foodstuffs Site 305m  
(approx 75% of extg box drain in Foodstuffs' site removed)



Project name : Papanui Pak'nSave  
Job no. : 171259/C/2  
Sketch title : Piped Option to Lydia St Drain  
Date : 24 Apr 2019  
Scale : 1:1000 @ A3  
Drawn : KGB  
Sketch : C42A



Lydia Drain Pipe Proposed Long Section (Scale V 1:50 H 1:1000)

Easement Width = 2 x depth to invert + OD (where OD = 860mm for 750Ø & 620mm for 525Ø)

Proposed concrete cover or class z pipe where cover is less than 600mm and pipe is in trafficked area

Proposed concrete cover or class z pipe where cover is less than 600mm and pipe is in trafficked area

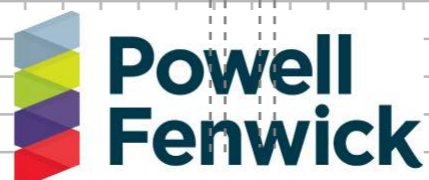
Boundary

Estimated locations 4 x Orion & one x Chorus buried service

Extg 375mm and headwall to be removed

Extg 900mmØ INV16.91 (estimated)

Possible services clashes outside boundary:  
 -Enable: (not affected)  
 -Chorus: 50mmØ crosses  
 -Orion: approx 4 cables will likely clash  
 -CCC: (sewer & water not affected)



Project name : Papanui Pak'nSave  
 Job no. : 171259/C/2  
 Sketch title : Lidia Street Drain Proposed Pipe route - Pipe Long Section  
 Date : 24 Apr 2019  
 Scale : 1:1000 hor and 1:50 vert at A3  
 Drawn : KGB  
 Sketch : C42B

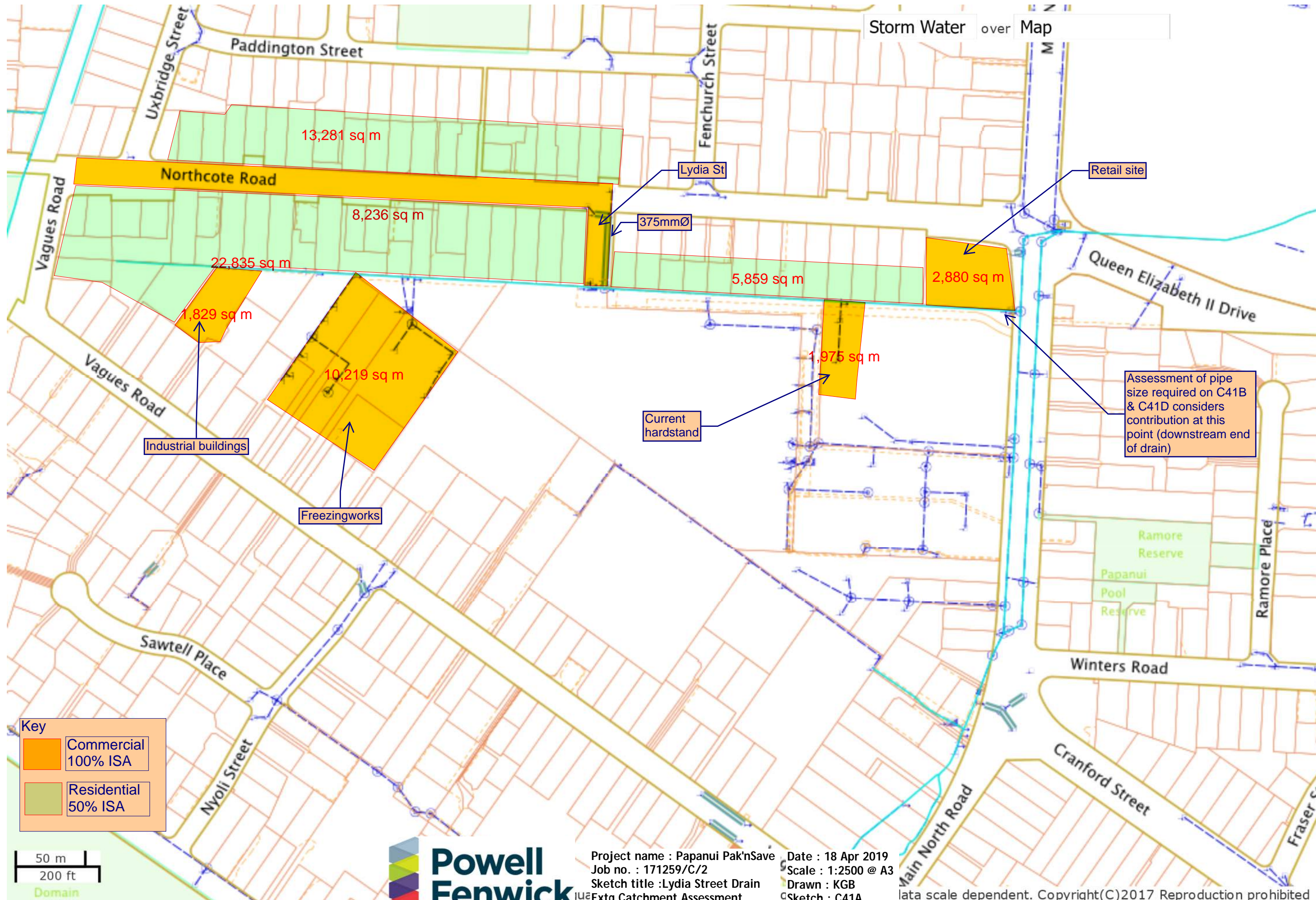




Table 1 Catchment Discharging to Lidia Street Drain			
	Areas		
	Residential	Commercial	
	13281	8236	
	22835	1829	
	5859	10219	
		1975	
		2880	
Totals	41975	25139	67114 Total area (m <sup>2</sup> )
C	0.5	1	0.69 Catchment Runoff coefficient

Total Contributing area of Lidia Drain at Main North Road Boundary

Christchurch Rainfall Intensities (Spreadsheet includes 16%)

Table 2 CCC Intensities mm/hr (includes 16% climate change allowance)										
Return Period	Duration									
	10min	20min	30min	1hr	2hr	6hr	12hr	18hr	24hr	30hr
5yr	41.0	28.9	23.6	16.7	11.8	6.8	4.8	3.9	3.4	2.9
10yr	49.4	34.9	28.4	20.1	14.2	8.2	5.8	4.7	4.1	3.5
20yr	58.1	41.0	33.4	23.6	16.6	9.6	6.8	5.5	4.8	4.1
50yr	70.3	49.6	40.5	28.6	20.2	11.6	8.2	6.7	5.8	4.9

Table 3 Flow due to catchment										
Return Period	Duration									
	10min	20min	30min	1hr	2hr	6hr	12hr	18hr	24hr	30hr
5yr	525.3	370.3	302.4	214.0	151.2	86.6	61.1	49.8	43.2	36.9
10yr	633.0	447.2	363.9	257.5	181.9	104.4	73.7	60.1	52.0	44.5
20yr	744.4	525.3	428.0	302.4	212.7	122.7	86.6	70.6	61.1	52.3
50yr	900.7	635.5	518.9	366.4	258.8	148.6	104.9	85.6	74.1	63.3

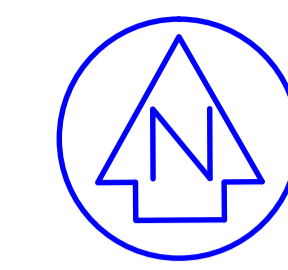
Design Storm 10yr 10 minute

For infrastructure design design storm is:

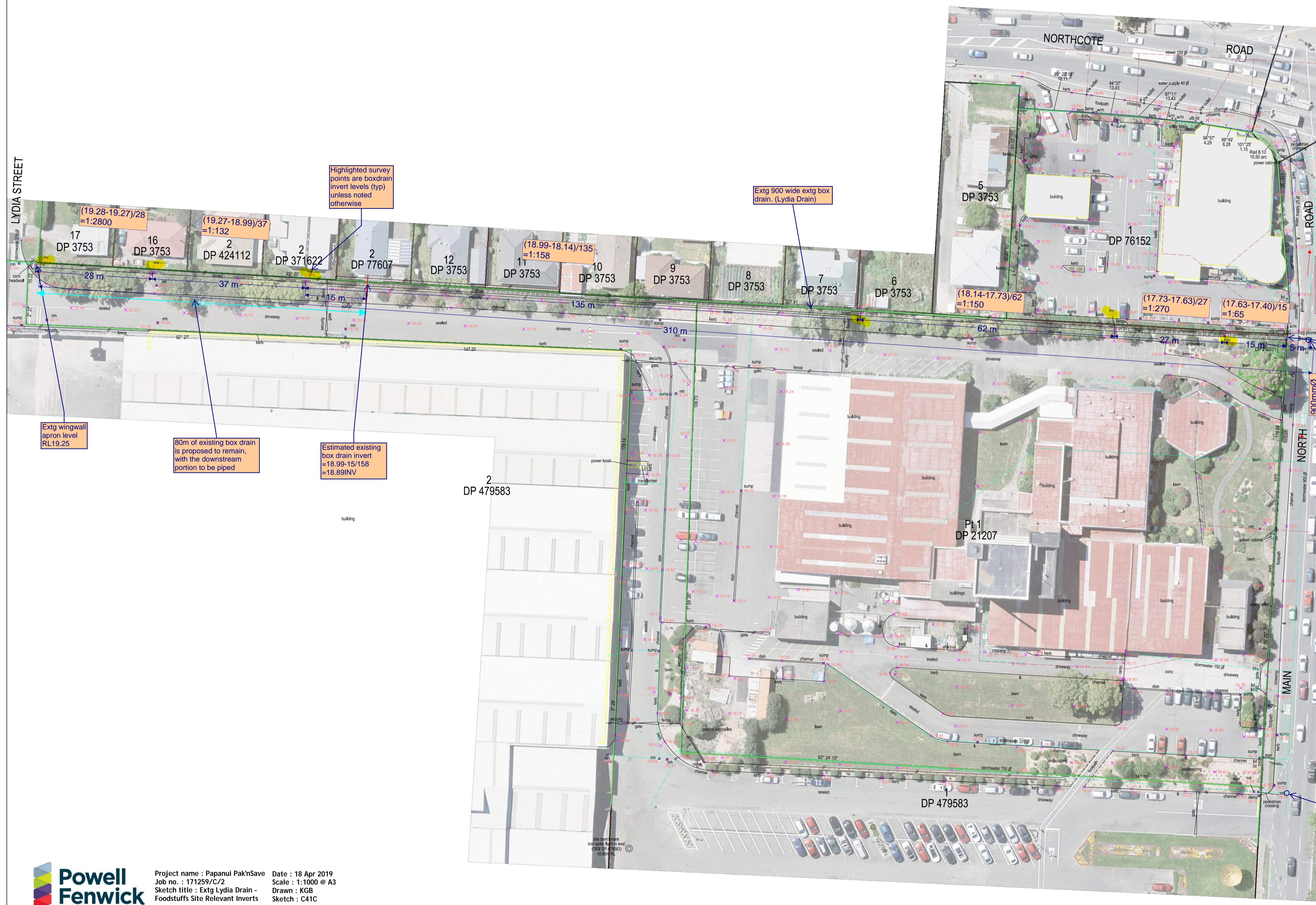
The primary system must cater for the more frequent rainfall events including the 20% AEP storm. The secondary system must convey over-design events without inundation hazard to house floors and building platforms at least to the 2% AEP storm, including occasions when there are blockages in the primary drainage system.

Have not calculated time of concentration but likely to be less than 10mins & certainly not less than 20

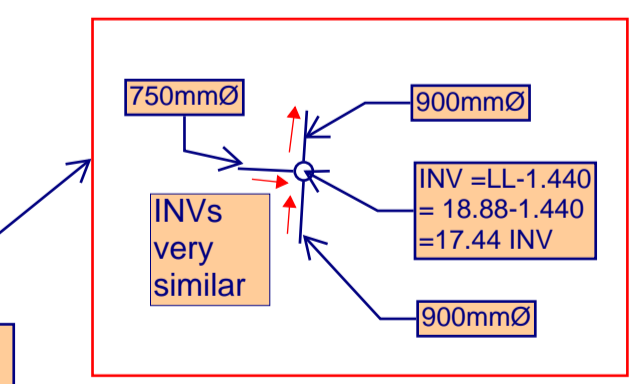
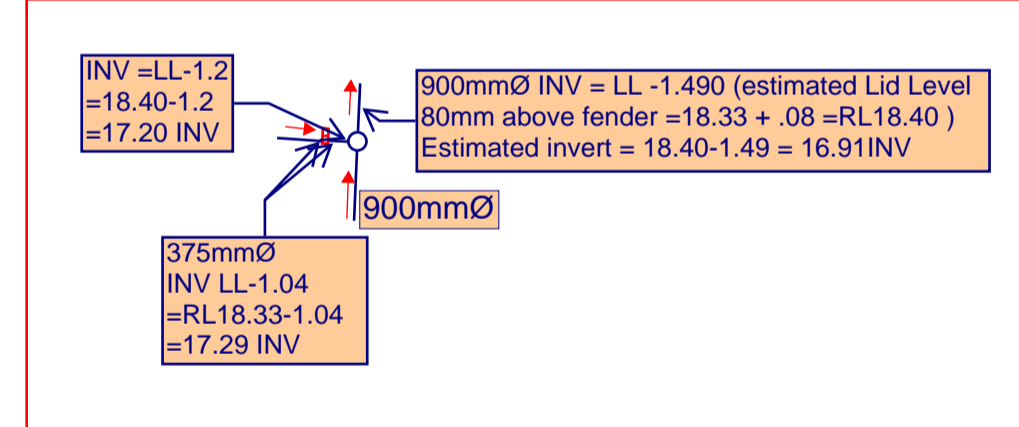
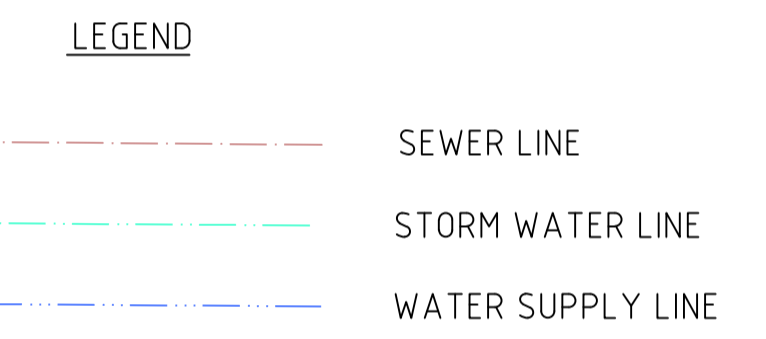
Time of concentration assessment



- NOTES :**
- THIS SURVEY HAS NOT INCLUDED SITE MARKING OF THE BOUNDARY POSITIONS UNLESS OTHERWISE INDICATED.
  - SOME UNDERGROUND DRAINAGE INFORMATION IS SHOWN ON THIS PLAN AND HAS BEEN SOURCED FROM COUNCIL DRAINAGE RECORDS. THE ACCURACY OF THIS INFORMATION IS NOT GUARANTEED AND THE LOCATION OF THE PIPEWORK REQUIRES ONSITE VERIFICATION.
  - THE SITE DATUM FOR THIS SURVEY IS:  
IRON SPIKE FLUSH IN SEAL  
REDUCED LEVEL: 18.86m  
ORIGIN OF LEVELS: BM133 (EKRJ)  
REDUCED LEVEL: 20.52m  
DATUM: CDD MARCH 2016
  - "SPOT HEIGHT" POSITIONS ARE SHOWN IN THE FOLLOWING FORMAT X AND ARE EXPRESSED IN METRES IN TERMS OF THE SITE DATUM FOR THE SURVEY. THE HEIGHTS OF THESE POSITIONS HAVE AN ACCURACY RELATIVE TO THE SITE DATUM AS FOLLOWS:  
    - X HARD SURFACES eg CONCRETE, ASPHALT +/- 0.030m @ 95% CONFIDENCE LEVEL.
    - X SOFT SURFACES eg GRASS +/- 0.050m @ 95% CONFIDENCE LEVEL.
  - CADASTRAL BOUNDARIES BEARINGS AND DISTANCES HAVE BEEN SOURCED FROM DP's 21207 & 479583 & SO's 19085 & 19672.
  - HEIGHTS SHOWN ON THE KERB LINES ARE ON TOP OF KERB.
  - HEIGHTS SHOWN FOR MANHOLES & SUMPS ARE ON TOP OF THOSE FEATURES.



-No surveyed INV has been provided for the existing 375mmØ RCRRJ ( internal dia, measured 380mm approx)  
-estimated invert= RL18.63-1.230 (measured 27 Sep 2018) 1.1=17.40 INV



**Powell Fenwick**  
 Project name : Papanui Pak'nSave Date : 18 Apr 2019  
 Job no. : 171259/C/2 Scale : 1:1000 @ A3  
 Sketch title : Extg Lydia Drain - Foodstuffs Site Relevant Inverts Drawn : KGB  
 Sketch : C41C



4 Meadow Street, PO Box 5558, Papanui, Christchurch  
 P 03 352 5599 AMBERLEY 03 314 9200  
 F 03 352 5527 ASHBURTON 03 307 7021  
 TOLL FREE 0508 787 887 DARFIELD 03 318 8151

REV	DATE	REVISION DETAILS	DESIGNED	VERIFIED
A	21/05/18	SITE SURVEY	S SMITH	

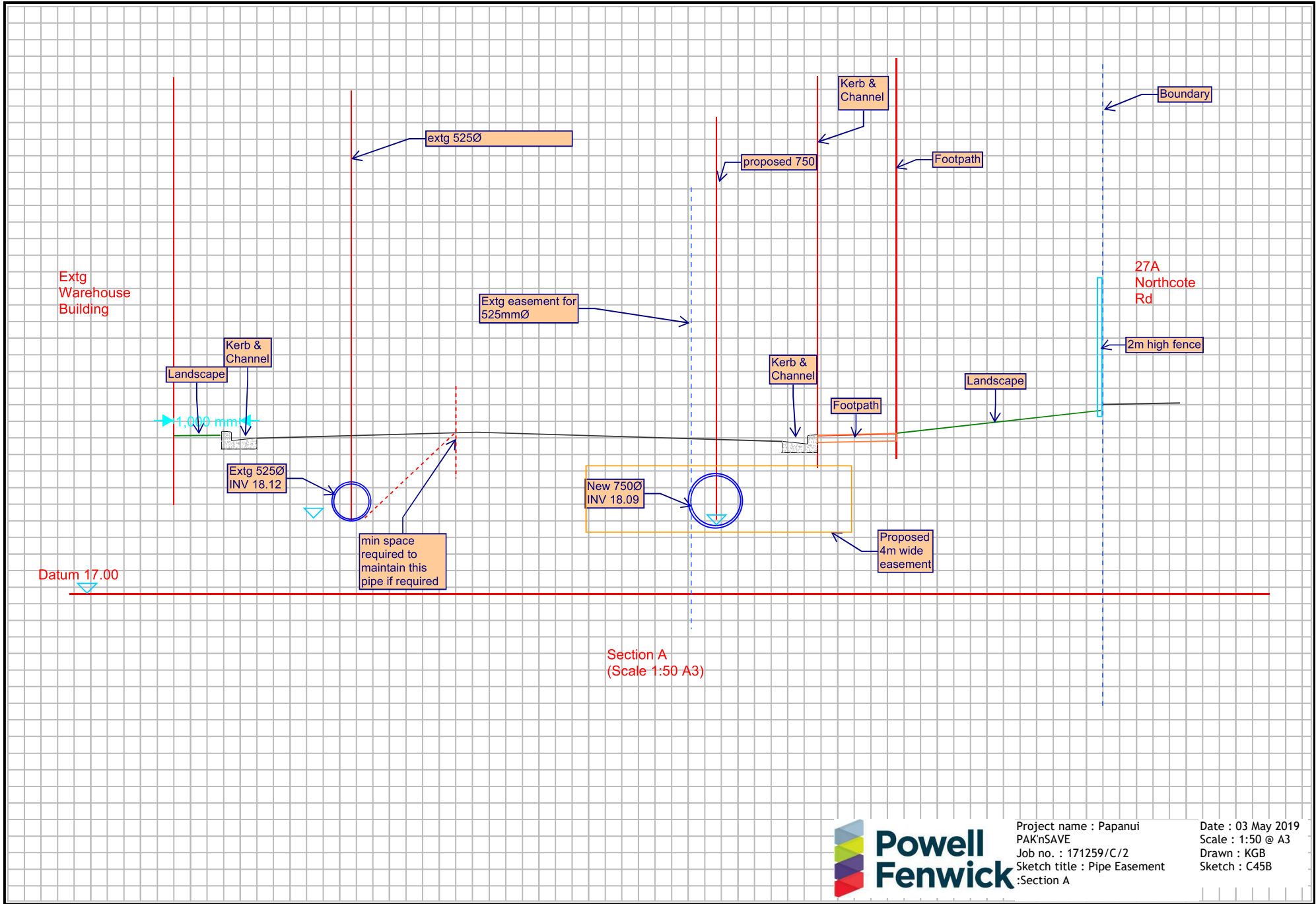
DESIGNED	VERIFIED
APPROVED	DATE

PROJECT		FOODSTUFFS S.I LTD - 171 MAIN NORTH ROAD	
TITLE		SITE SURVEY	
SHEET 1 OF 2			

INFORMATION ONLY	
PROJECT NO 13836	
SCALE 1:500 (A1)	SIZE A1
DRAWING NO SS-01	REV A

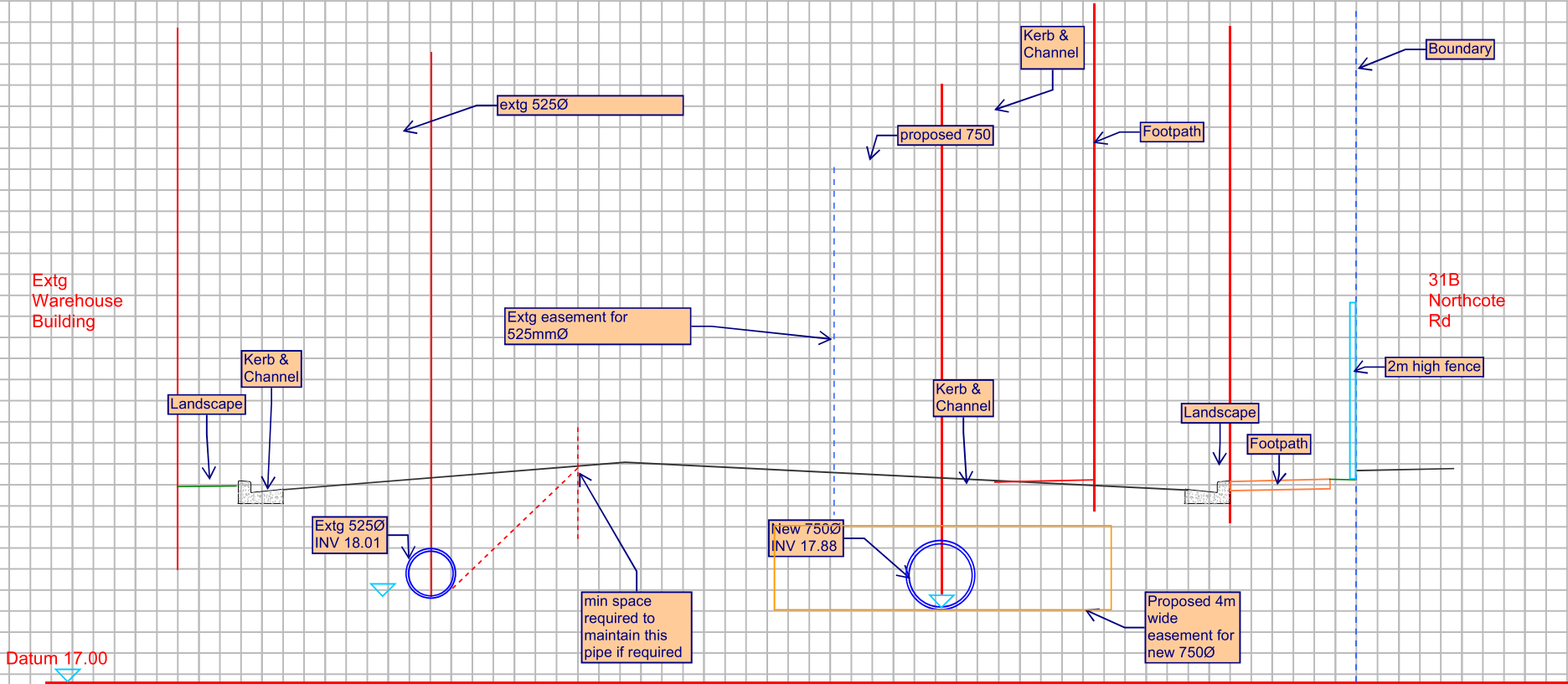






Project name : Papanui  
 PAK'nSAVE  
 Job no. : 171259/C/2  
 Sketch title : Pipe Easement  
 :Section A

Date : 03 May 2019  
 Scale : 1:50 @ A3  
 Drawn : KGB  
 Sketch : C45B



Section B  
(Scale 1:50 A3)



Project name : Papanui  
 PAK'nSAVE  
 Job no. : 171259/C/2  
 Sketch title : Pipe Easement:  
 Section B

Date : 03 May 2019  
 Scale : 1:50 @ A3  
 Drawn : KGB  
 Sketch : C45C